

Traffic Analysis

Proposed Grangeville Avenue / BNSF Railroad Grade Separation

Hanford, California

Prepared For:

TRC Companies, Inc.
10680 White Rock Road, Suite 100
Rancho Cordova, California 95670

Date:

April 30, 2019

Job No.:

18-066.01



PETERS ENGINEERING GROUP

A CALIFORNIA CORPORATION

Mr. Mark Imbriani
TRC Companies, Inc.
10680 White Rock Road, Suite 100
Rancho Cordova, California 95670

April 30, 2019

Subject: Traffic Analysis
Proposed Grangeville Avenue / BNSF Railroad Grade Separation
Hanford, California

Dear Mr. Imbriani:

INTRODUCTION

This report presents the results of traffic counts and analyses related to construction of the Grangeville Avenue / BNSF Railroad grade separation in Hanford, California. This limited analysis focuses on the traffic volumes that will be diverted by the project and the anticipated conditions after completion of the project.

PROJECT DESCRIPTION

The proposed Grangeville Avenue / BNSF Railroad Grade Separation will be an underpass below the railroad tracks that will likely affect at least 850 feet in each direction along Grangeville Avenue.

Grangeville Boulevard is an arterial street that will be closed during construction, likely between University Avenue and Rodgers Road. It is anticipated that the intersection of Rodgers Road and Grangeville Boulevard will require full closure during at least a portion of the construction schedule. The following locations will be closed during construction and the connections will likely be permanently eliminated:

- Mildred Street at Grangeville Boulevard;
- Santa Fe Mini Storage Driveway;
- Tara Mobile Estates Driveway.

The project may include improvements on Claridge Lane to provide access to the existing mini storage facility and a new driveway on Malone Street to provide access to the mobile estates.

A vicinity map is presented in the attached Figure 1, Site Vicinity Map.

EXISTING TRAFFIC VOLUMES

Intersection turning movement traffic counts were performed on a weekday between 7:00 and 9:00 a.m. and between 4:00 and 6:00 p.m. at the following intersections:

- University Avenue / Grangeville Avenue

- Tara Mobile Homes / Grangeville Avenue
- Mildred Street / Santa Fe Central & Mini Storage / Grangeville Avenue
- Rodgers Road / Grangeville Avenue

The intersection counts included bicycles, pedestrians, and heavy vehicles. The traffic count data sheets are attached and include the dates the counts were performed. The existing peak-hour turning movement volumes are presented in Figure 2, Existing Peak Hour Traffic Volumes.

A 24-hour vehicle classification count was performed on Grangeville Avenue near the BNSF crossing on Tuesday, December 18, 2018. The count revealed a total 24-hour traffic volume (eastbound and westbound combined) of 14,166 vehicles with a peak-hour volume of 1,336 vehicles between 4:30 and 5:30 p.m. Approximately five percent of the vehicles are trucks (heavy vehicles) of two or more axles.

LANE CONFIGURATIONS AND INTERSECTION CONTROL

The existing lane configurations and intersection control at the study intersections are illustrated in Figure 3, Existing Lane Configurations.

TRAFFIC VOLUMES AT ALTERNATE CROSSINGS

The nearest alternate locations to cross the BNSF railroad exist on Fargo Avenue to the north and Elm Street to the south. 11th Avenue also provides an alternative north-south crossing south of Elm Street, and Lacey Boulevard is the next major east-west street to the south. It is assumed that the detour established for the closure will include 12th Avenue, Fargo Avenue, and 11th Avenue. Traffic is also likely to redistribute to Greenfield Avenue and Elm Street to cross the tracks, or to 11th Avenue south of Elm Street. The following is a description of the alternate crossings:

Fargo Avenue at the BNSF Railroad is a two-lane at-grade crossing (one lane in each direction) with active traffic control devices, post-mounted and cantilevered flashing lights, signage, pavement markings, raised medians, and two automatic gates on each approach. The crossing is approximately one mile north of Grangeville Avenue. Fargo Avenue is designated as an arterial street in the City of Hanford General Plan with a current traffic volume on the order of 10,500 vehicles per day (both directions combined) based on traffic count data provided by the City of Hanford.

Elm Street at the BNSF Railroad is a four-lane at-grade crossing (two lanes in each direction) with active traffic control devices, post-mounted and cantilevered flashing lights, signage, pavement markings, and one automatic gate on each approach. There are no raised medians at the crossing. The crossing is approximately 0.7 mile south of Grangeville Avenue. Elm Street is a local road with an existing traffic volume on the order of 6,800 vehicles per day (both directions combined) based on traffic count data provided by the City of Hanford. The stop-controlled intersection of Elm Street and Greenfield Avenue is located approximately 350 feet west of the at-grade crossing. The signalized intersection of Elm Street and 11th Avenue is located less than 300 feet east of the at-grade crossing.

Lacey Boulevard at the BNSF Railroad is a four-lane at-grade crossing (two lanes in each direction) with active traffic control devices, post-mounted flashing lights, signage, pavement markings, raised medians, and two automatic gates on each approach. The crossing is approximately one mile south and half a mile east of the Grangeville Avenue crossing. Lacey Boulevard is designated as a collector street east of 11th Avenue in the City of Hanford General Plan with a current traffic volume on the order of 11,400 vehicles per day (both directions combined) based on traffic count data provided by the City of Hanford.

ROAD SEGMENT ANALYSES

The Transportation Research Board *Highway Capacity Manual*, 2000, (HCM) defines level of service (LOS) as a qualitative measure describing operational characteristics within a traffic stream, based on service measures such as speed and travel time, freedom to maneuver, traffic interruptions, comfort, and convenience. Level-of-service characteristics for road segments are presented in Table 1.

Table 1
Level of Service Characteristics for Roadways

Level of Service	Description
A	Primarily free flow operations
B	Reasonably unimpeded operations, ability to maneuver only slightly restricted
C	Stable operations, ability to maneuver and select operating speed affected
D	Unstable flow, speeds and ability to maneuver restricted
E	Significant delays, flow quite unstable
F	Extremely slow speeds

Reference: 1998 *Highway Capacity Manual*, Transportation Research Board

For general planning purposes, road segment levels of service were determined based on procedures outlined in the HCM2010 utilizing the 2012 Florida Department of Transportation (FDOT) Quality/Level of Service Handbook Tables (Florida tables). The Florida tables present generalized correlations between traffic volumes and LOS based on the nationally-utilized and accepted HCM2010; the Florida tables are frequently utilized throughout California for road segment analyses. The Florida tables present LOS criteria based on the type of roadway being analyzed and the regional setting (i.e., urban areas or transitioning areas). The applicable Florida table is attached.

It should be noted that the actual operations of the roadways will likely be governed by the operations at intersections near the crossings. The analyses presented herein should be utilized only for the discussion of the order of magnitude of the effects of the Grangeville Avenue closure.

Table 2 presents the specific volume thresholds used in the analyses. It should be noted that reference to “signalized” roadways in the Florida tables includes stop-controlled intersections.

Table 2
Volume Thresholds for Non-State Signalized Roadway Levels of Service

Lanes	Configuration	A	B	C	D	E/F
2	Divided, ≥40 MPH (Fargo Avenue)	*	*	≤15,876	15,877 – 16,726	>16,726
4	Undivided, ≤35 MPH (Elm Street)	*	*	≤9,787	9,788 – 21,870	>21,870
4	Divided, ≤35 MPH (Lacey Boulevard)	*	*	≤13,050	13,050 – 30,420	>30,420

Reference: Florida Department of Transportation Table 1, Generalized Annual Average Daily Volumes for Florida's Urbanized Areas (utilizing Non-State Signalized Roadway Adjustments) dated December 18, 2012

Table 3 presents the results of the road segment analyses. The analyses are based on an assumption that 50 percent of the existing trips on Grangeville Avenue will redistribute to Fargo Avenue, 25 percent will use Elm Street, and 25 percent will use Lacey Boulevard.

Table 3
Summary of Estimated LOS

Road Segment	Existing Volume	Existing LOS	Detour Volume	Detour LOS
Fargo Avenue	10,500	C or better	17,583	E/F
Elm Street	6,800	C or better	10,342	D
Lacey Boulevard	11,400	C or better	14,942	D

DISCUSSION OF ANALYSES

The results of the road segment analyses suggest that congestion and delays are likely on Fargo Avenue and on Elm Street during construction. The conditions on Elm Street are expected to be worse than suggested by the road segment analyses because of the short length of the road segment and the proximity of intersections to the crossing. Congested conditions at intersections along the detour route should be anticipated.

A public information campaign is recommended to alert motorists of the project, the alternate routes, and the potential for congestion. It is recommended that alternate routes farther from the project site, such as State Route 198 and Flint Avenue, be suggested as alternate routes.

CONCLUSIONS

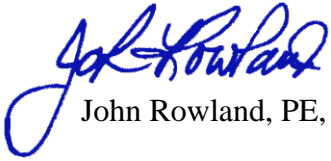
Standard traffic engineering principles and methods were employed to study the existing conditions and to estimate conditions that may occur during construction.

The conclusion of this study is that the nearest potential detour routes available during construction of the Grangeville Avenue / BNSF grade separation are likely to experience severe congestion and delays during construction.

A public information campaign is recommended to alert motorists of the project, the alternate routes, and the potential for congestion. It is recommended that alternate routes farther from the project site, such as State Route 198 and Flint Avenue, be suggested as alternate routes.

Thank you for the opportunity to perform this traffic analysis. Please feel free to call our office if you have any questions.

PETERS ENGINEERING GROUP



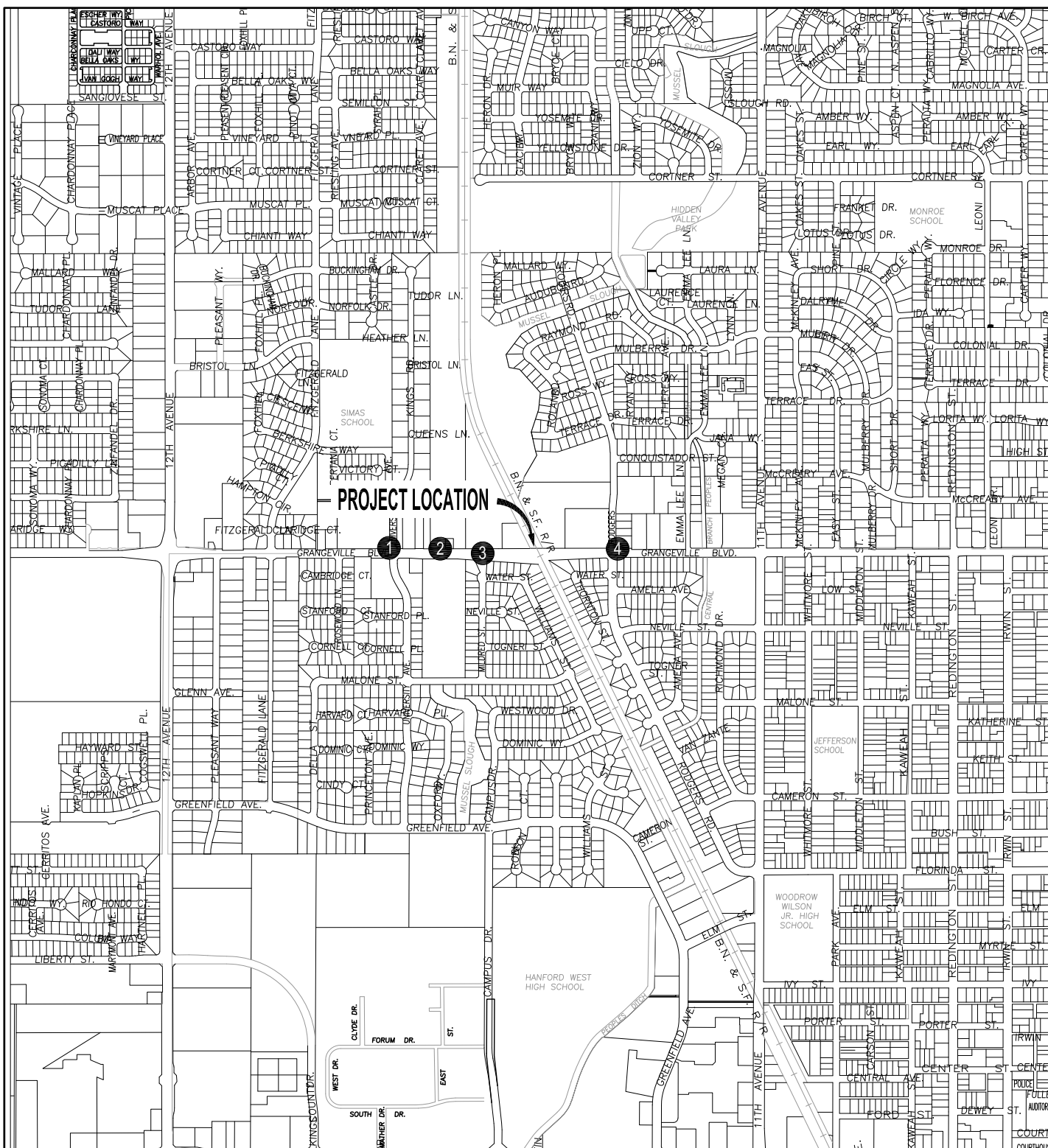
John Rowland, PE, TE



Attachments: Figures 1 through 3
Traffic Count Data Sheets
Florida Table



FIGURES



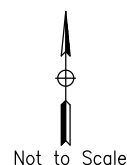


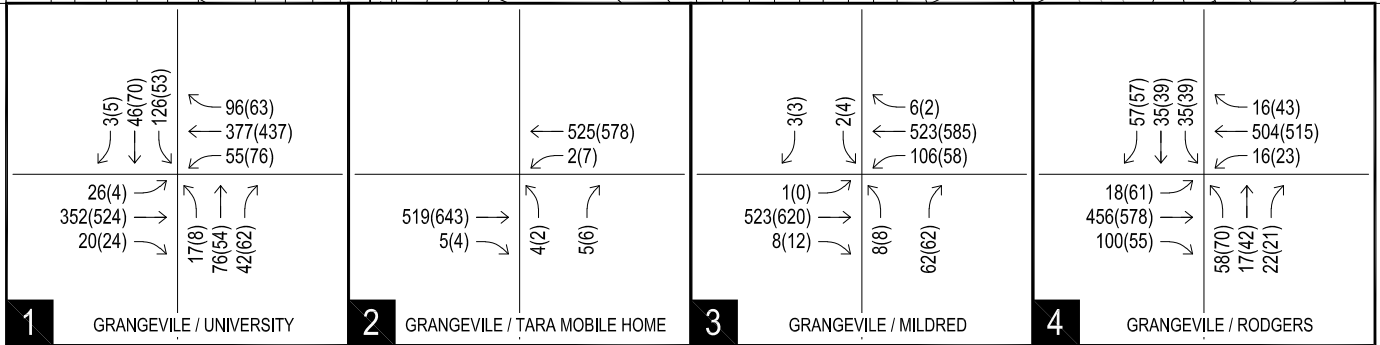
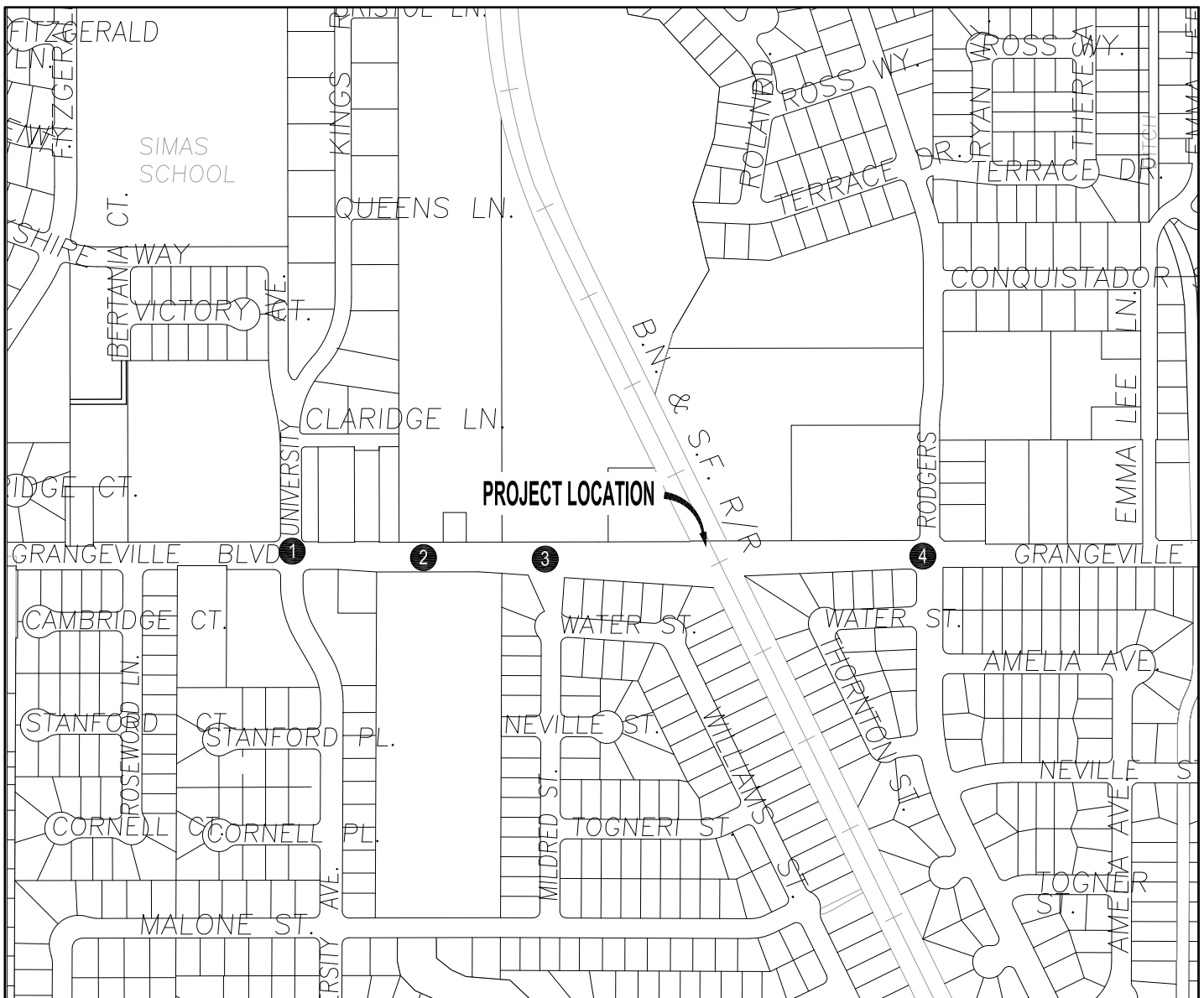
Proposed Grangeville Avenue / BNSF Railroad Grade Separation Hanford, California

LEGEND

-  STUDY AREA INTERSECTIONS
-  PROJECT SITE



SITE VICINITY MAP



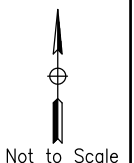


Proposed Grangeville Avenue / BNSF Railroad Grade Separation Hanford, California

LEGEND

-  STUDY AREA INTERSECTIONS
- XX (YY) AM (PM) PEAK HOUR VOLUMES
-  PROJECT SITE

EXISTING PEAK HOUR TRAFFIC VOLUMES

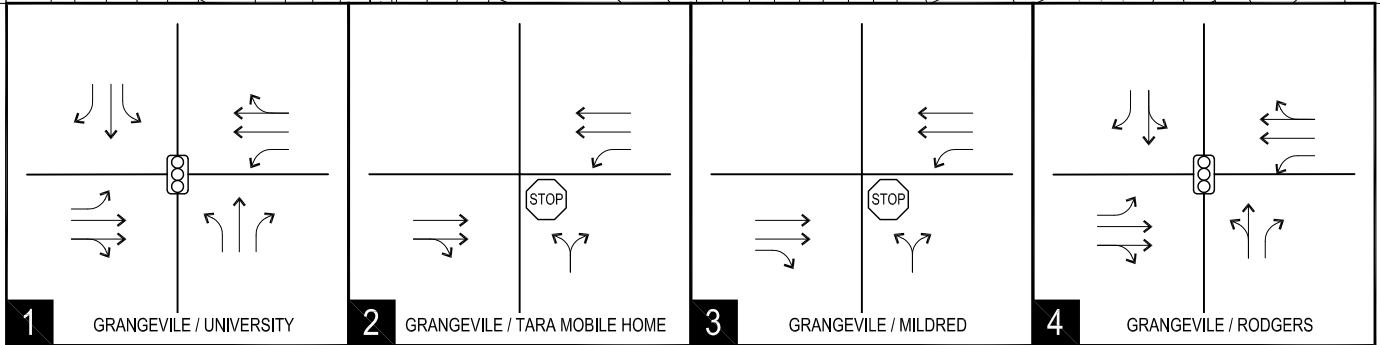
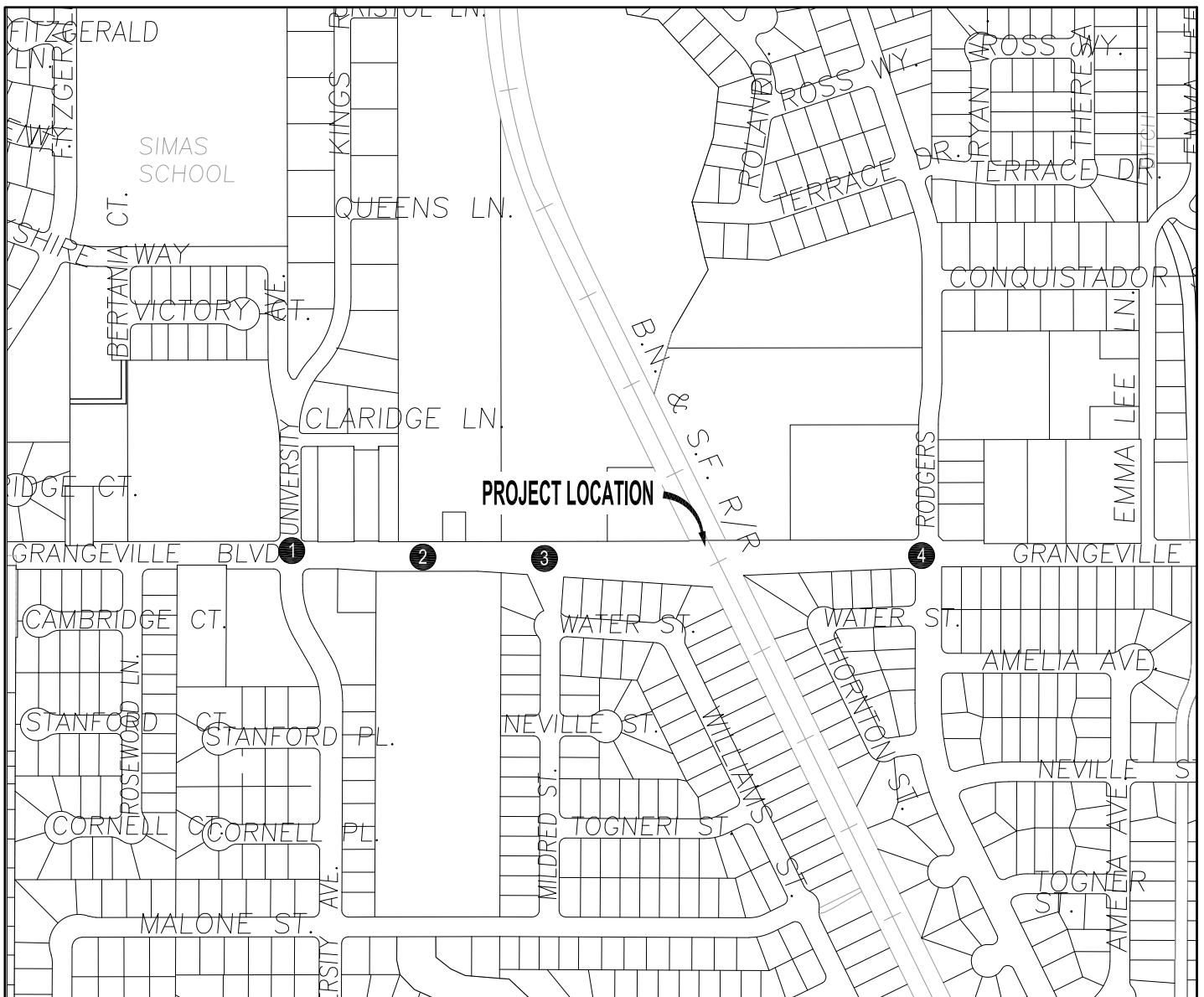


Not to Scale



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Figure 2



Proposed Grangeville Avenue / BNSF Railroad Grade Separation Hanford, California

LEGEND

- STUDY AREA INTERSECTIONS
- PROJECT SITE
- SIGNALIZED INTERSECTION
- STOP SIGN
- DIRECTION OF TRAVEL

EXISTING LANE CONFIGURATIONS AND STOP CONTROL



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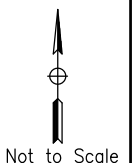


Figure 3

TRAFFIC COUNT DATA SHEETS





Metro Traffic Data Inc.
310 N. Irwin Street - Suite 20
Hanford, CA 93230
800-975-6938 Phone/Fax
www.metrotrafficdata.com

Turning Movement Report

Prepared For:

Peters Engineering Group
952 Pollasky Avenue
Clovis, CA 93612

LOCATION Grangeville Blvd @ University Ave

LATITUDE 36.3426

COUNTY Kings

LONGITUDE -119.6661

COLLECTION DATE Tuesday, December 18, 2018

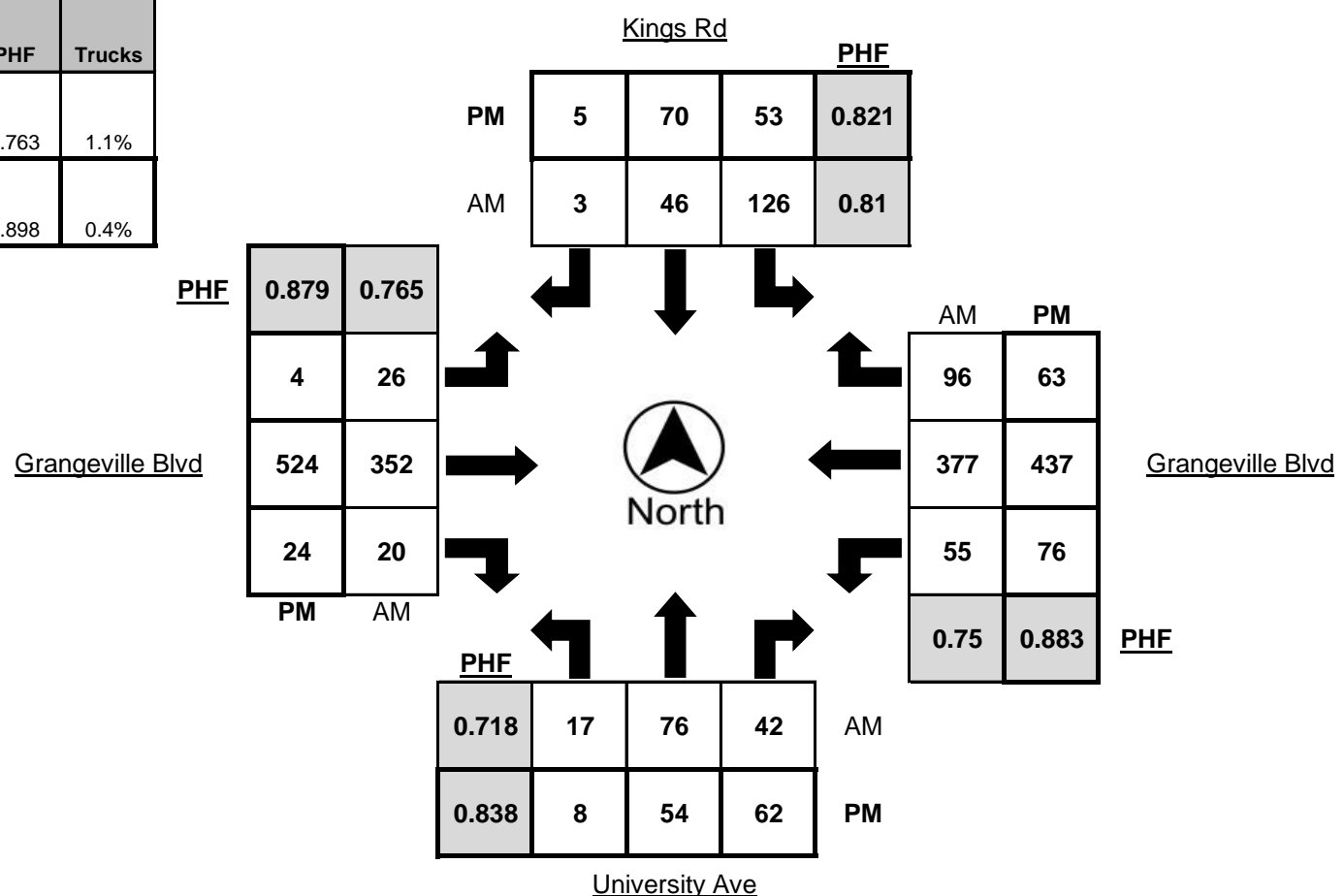
WEATHER AM Fog / PM Clear

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	0	7	3	1	10	12	0	1	1	34	2	0	4	50	6	4
7:15 AM - 7:30 AM	3	11	9	0	25	6	0	1	4	59	4	0	10	63	11	4
7:30 AM - 7:45 AM	7	26	14	0	38	13	2	1	11	100	5	0	12	88	27	0
7:45 AM - 8:00 AM	3	32	10	1	42	11	1	0	10	113	7	1	18	110	48	0
8:00 AM - 8:15 AM	4	7	9	0	21	16	0	0	1	80	4	1	15	116	10	4
8:15 AM - 8:30 AM	1	2	6	1	13	14	2	0	1	78	5	0	3	57	11	2
8:30 AM - 8:45 AM	2	3	3	0	7	17	1	0	0	67	3	1	11	55	7	1
8:45 AM - 9:00 AM	3	5	3	0	9	11	0	0	0	64	2	2	10	73	6	4
TOTAL	23	93	57	3	165	100	6	3	28	595	32	5	83	612	126	19

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	3	12	8	0	12	13	2	0	1	105	1	0	11	110	5	2
4:15 PM - 4:30 PM	3	6	6	0	18	10	1	0	0	125	7	1	8	104	12	1
4:30 PM - 4:45 PM	2	13	22	0	13	17	1	0	1	119	4	2	14	103	9	0
4:45 PM - 5:00 PM	2	11	12	0	14	12	2	0	3	105	8	0	18	93	15	1
5:00 PM - 5:15 PM	1	16	19	0	12	17	1	0	0	148	7	0	27	114	22	0
5:15 PM - 5:30 PM	3	14	9	0	14	24	1	0	0	152	5	0	17	127	17	2
5:30 PM - 5:45 PM	1	17	10	0	7	8	2	0	2	107	5	0	12	110	12	0
5:45 PM - 6:00 PM	1	11	8	0	14	7	1	0	0	111	3	0	4	84	17	0
TOTAL	16	100	94	0	104	108	11	0	7	972	40	3	111	845	109	6

PEAK HOUR	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	17	76	42	1	126	46	3	2	26	352	20	2	55	377	96	8
4:30 PM - 5:30 PM	8	54	62	0	53	70	5	0	4	524	24	2	76	437	63	3

	PHF	Trucks
AM	0.763	1.1%
PM	0.898	0.4%





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Turning Movement Report

Prepared For:

Peters Engineering Group
952 Pollasky Avenue
Clovis, CA 93612

LOCATION Grangeville Blvd @ Mildred St

LATITUDE 36.3426

COUNTY Kings

LONGITUDE -119.6636

COLLECTION DATE Tuesday, December 18, 2018

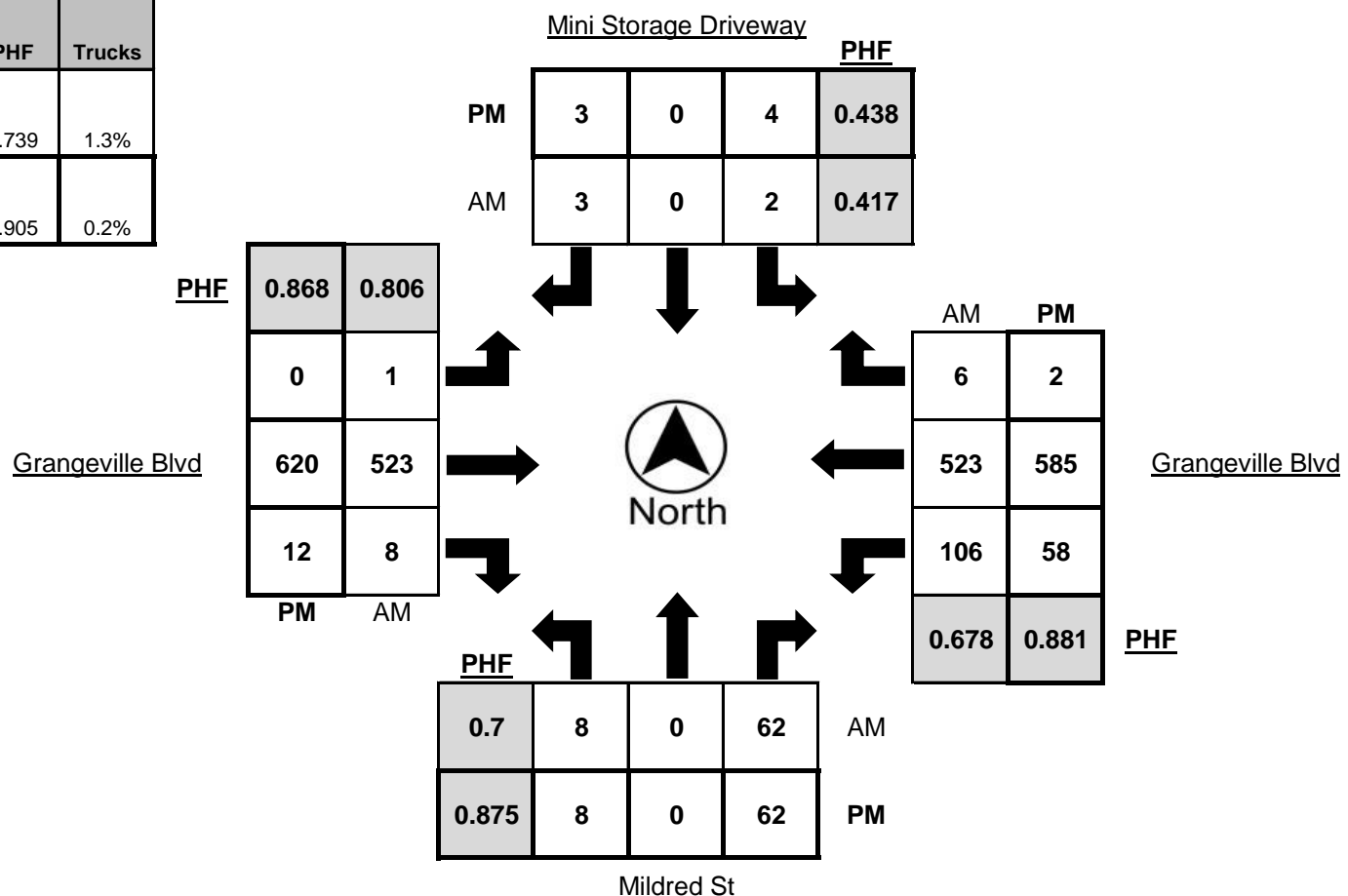
WEATHER AM Fog / PM Clear

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	2	0	10	1	0	0	1	0	0	48	1	2	3	56	3	4
7:15 AM - 7:30 AM	2	0	17	1	0	0	1	0	0	95	0	1	11	83	1	4
7:30 AM - 7:45 AM	1	0	24	1	0	0	1	0	0	148	3	1	41	131	1	0
7:45 AM - 8:00 AM	4	0	17	0	0	0	0	0	0	162	3	2	36	197	1	3
8:00 AM - 8:15 AM	1	0	4	0	2	0	1	1	1	118	2	1	18	112	3	1
8:15 AM - 8:30 AM	2	0	8	0	1	0	0	0	0	94	1	1	9	68	1	3
8:30 AM - 8:45 AM	0	0	6	0	1	0	0	0	1	78	1	1	5	76	0	1
8:45 AM - 9:00 AM	2	0	4	0	0	0	0	0	0	71	2	1	3	85	3	4
TOTAL	14	0	90	3	4	0	4	1	2	814	13	10	126	808	13	20

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	2	0	12	0	2	0	1	0	0	120	3	0	12	124	3	2
4:15 PM - 4:30 PM	2	0	11	0	1	0	0	0	0	144	2	0	10	122	0	1
4:30 PM - 4:45 PM	2	0	16	0	1	0	1	0	0	150	1	0	16	127	1	1
4:45 PM - 5:00 PM	2	0	13	0	0	0	0	0	0	124	3	0	13	136	1	1
5:00 PM - 5:15 PM	2	0	18	0	3	0	1	0	0	180	2	0	12	156	0	1
5:15 PM - 5:30 PM	2	0	15	0	0	0	1	0	0	166	6	0	17	166	0	0
5:30 PM - 5:45 PM	2	0	10	0	0	0	0	0	0	123	1	0	13	137	0	0
5:45 PM - 6:00 PM	1	0	11	0	1	0	0	0	2	132	2	0	10	104	0	1
TOTAL	15	0	106	0	8	0	4	0	2	1139	20	0	103	1072	5	7

PEAK HOUR	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	8	0	62	2	2	0	3	1	1	523	8	5	106	523	6	8
4:30 PM - 5:30 PM	8	0	62	0	4	0	3	0	0	620	12	0	58	585	2	3

	PHF	Trucks
AM	0.739	1.3%
PM	0.905	0.2%





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LATITUDE 36.3427

COUNTY Kings

LONGITUDE -119.6596

COLLECTION DATE Tuesday, December 18, 2018

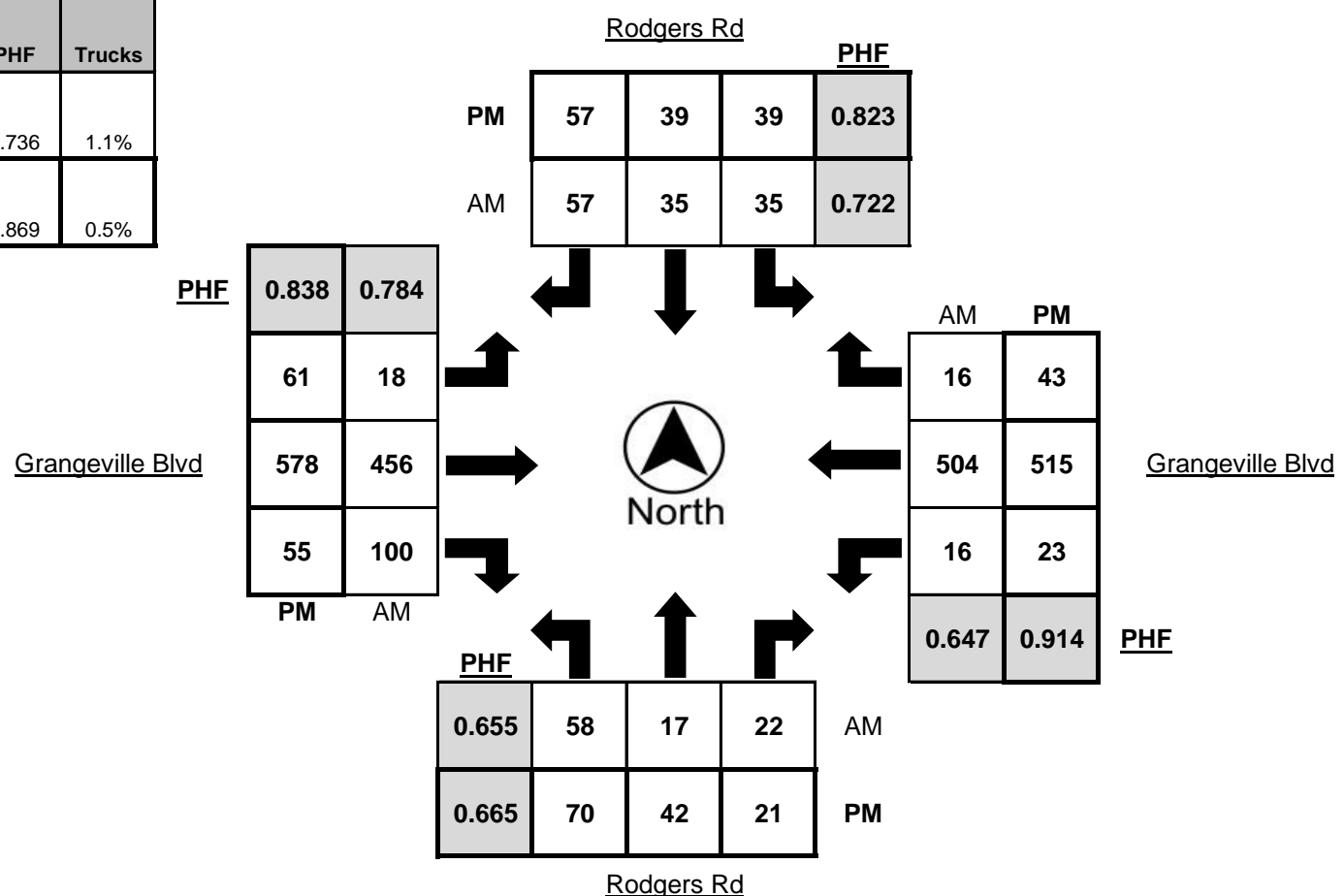
WEATHER AM Fog / PM Clear

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:00 AM - 7:15 AM	3	0	4	1	6	8	7	1	2	48	5	2	3	49	3	3
7:15 AM - 7:30 AM	8	2	4	0	7	7	16	0	1	97	13	1	2	72	4	4
7:30 AM - 7:45 AM	15	5	4	0	13	14	17	0	5	135	28	2	3	130	2	0
7:45 AM - 8:00 AM	23	3	11	2	7	8	11	0	4	142	37	2	6	196	5	3
8:00 AM - 8:15 AM	12	7	3	0	8	6	13	0	8	82	22	0	5	106	5	1
8:15 AM - 8:30 AM	9	1	3	0	6	3	6	0	5	90	12	1	0	58	5	3
8:30 AM - 8:45 AM	8	1	2	0	4	8	3	0	4	73	7	1	1	70	2	1
8:45 AM - 9:00 AM	8	5	2	0	4	9	7	0	5	65	6	2	1	76	2	4
TOTAL	86	24	33	3	55	63	80	1	34	732	130	11	21	757	28	19

Time	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
4:00 PM - 4:15 PM	9	9	5	0	5	9	8	0	8	116	13	0	2	126	9	2
4:15 PM - 4:30 PM	11	6	6	0	5	8	5	1	10	128	14	1	5	118	4	1
4:30 PM - 4:45 PM	13	7	4	1	11	16	14	1	10	132	16	1	4	120	10	0
4:45 PM - 5:00 PM	11	13	0	0	5	7	12	0	20	119	12	1	5	124	8	1
5:00 PM - 5:15 PM	27	12	11	0	12	11	13	0	19	176	12	0	10	129	12	2
5:15 PM - 5:30 PM	19	10	6	0	11	5	18	0	12	151	15	0	4	142	13	0
5:30 PM - 5:45 PM	17	9	5	0	4	6	7	0	17	107	5	0	5	120	12	0
5:45 PM - 6:00 PM	17	10	4	0	7	8	9	0	12	118	8	0	5	90	5	0
TOTAL	124	76	41	1	60	70	86	2	108	1047	95	3	40	969	73	6

PEAK HOUR	Northbound				Southbound				Eastbound				Westbound			
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks
7:15 AM - 8:15 AM	58	17	22	2	35	35	57	0	18	456	100	5	16	504	16	8
4:30 PM - 5:30 PM	70	42	21	1	39	39	57	1	61	578	55	2	23	515	43	3

	PHF	Trucks
AM	0.736	1.1%
PM	0.869	0.5%



[illegible]

Traffic Counts

CITY OF HANFORD

(update counts every 3 years)

Street	Location	Station	2012 Traffic Count	2013 Traffic Count	2014 Traffic Count	2015 Traffic Count	2016 Traffic Count	2017 Traffic Count	2018 Traffic Count
Douty St.	south of Lang	109			2,766			2,828	
Elm St.	west of 11th	65						6,831	
Fargo Ave.	west of 12th	155		3,381			3,587		3,708
	east of 12th	35		7,868			8,977		9,249
	west of Fountain Plaza	12	9,459			10,502			10,451
	east of Aspen	13			9,314			9,961	
	east of Kensington	14		8,216			8,187		8,651
	west of Encore	15		4,732			4,661		4,972
	west of 9 1/4	16			2,743			3,068	
Fifth St.	east of Brown	104	765				977		
Fitzgerald Ln	south of Castoro	163	2,334				1,980		
	south of Bristol	30		3,713			3,204		
Flint Ave	west of 11th	28		1,968			3,138		
	west of Douty	3	3,770				4,568		
	west of Hwy 43	11		3,889			5,020		5,165
Florinda St.	west of Kaweah	61			4,922			5,282	
	east of Brown	62			4,768			5,115	
	west of Gladys	63	3,199				4,684		

Traffic Counts

CITY OF HANFORD

(update counts every 3 years)

Street	Location	Station	2012 Traffic Count	2013 Traffic Count	2014 Traffic Count	2015 Traffic Count	2016 Traffic Count	2017 Traffic Count	2018 Traffic Count
Hume Ave.	west of Dawn	131	2,651				3,083		
	east of Santa Rosa	147	2,270				1,072		
Idaho Ave.	east of 11th	143	512				658		
Iona Ave.	east of 11th	139	723				1,041		
Irwin St.	north of Katherine	52	2,038			1,865			1,994
	north of Myrtle	75	3,789			3,249			3,274
	north of Seventh	93		2,886			2,885		
	south of Han/Arm	129		1,402			949		
Ivy St.	west of Kaweah	67		2,333			2,605		NEED
	east of Brown	68		1,843			1,853		NEED
Kings Co. Dr	south of Forum	82			3,370			3,373	
Lacey Blvd.	west of 13th	98			7,221			7,634	
	east of Magna Carta	77			12,246			11,535	
	west of 12th	78			13,105			11,772	
	east of Mall	79			15,829			15,648	
	west of Greenfield	80			16,211			17,448	
	west of Phillips	81			9,075			11,391	
	west of 9 1/2	84	4,753		7,003			6,982	

FLORIDA TABLE

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**Generalized Annual Average Daily Volumes for Florida's
Urbanized Areas**

TABLE 1

12/18/12

INTERRUPTED FLOW FACILITIES						UNINTERRUPTED FLOW FACILITIES					
STATE SIGNALIZED ARTERIALS						FREEWAYS					
Class I (40 mph or higher posted speed limit)						Core Urbanized					
Lanes	Median	B	C	D	E	Lanes	B	C	D	E	
2	Undivided	*	16,800	17,700	**	4	47,400	64,000	77,900	84,600	
4	Divided	*	37,900	39,800	**	6	69,900	95,200	116,600	130,600	
6	Divided	*	58,400	59,900	**	8	92,500	126,400	154,300	176,600	
8	Divided	*	78,800	80,100	**	10	115,100	159,700	194,500	222,700	
						12	162,400	216,700	256,600	268,900	
Class II (35 mph or slower posted speed limit)						Urbanized					
Lanes	Median	B	C	D	E	Lanes	B	C	D	E	
2	Undivided	*	7,300	14,800	15,600	4	45,800	61,500	74,400	79,900	
4	Divided	*	14,500	32,400	33,800	6	68,100	93,000	111,800	123,300	
6	Divided	*	23,300	50,000	50,900	8	91,500	123,500	148,700	166,800	
8	Divided	*	32,000	67,300	68,100	10	114,800	156,000	187,100	210,300	
Non-State Signalized Roadway Adjustments						Freeway Adjustments					
(Alter corresponding state volumes by the indicated percent.)						Auxiliary Lanes					
Non-State Signalized Roadways						Present in Both Directions					
						+ 20,000					
Median & Turn Lane Adjustments						Ramp Metering					
						+ 5%					
Lanes	Median	Exclusive Left Lanes	Exclusive Right Lanes	Adjustment Factors		UNINTERRUPTED FLOW HIGHWAYS					
2	Divided	Yes	No	+5%		Lanes	Median	B	C	D	E
2	Undivided	No	No	-20%		2	Undivided	8,600	17,000	24,200	33,300
Multi	Undivided	Yes	No	-5%		4	Divided	36,700	51,800	65,600	72,600
Multi	Undivided	No	No	-25%		6	Divided	55,000	77,700	98,300	108,800
-	-	-	Yes	+ 5%		Uninterrupted Flow Highway Adjustments					
One-Way Facility Adjustment						Lanes	Median	Exclusive left lanes	Adjustment factors		
Multiply the corresponding two-directional volumes in this table by 0.6						2	Divided	Yes	+5%		
						Multi	Undivided	Yes	-5%		
						Multi	Undivided	No	-25%		
BICYCLE MODE²						¹ Values shown are presented as two-way annual average daily volumes for levels of service and are for the automobile/truck modes unless specifically stated. This table does not constitute a standard and should be used only for general planning applications. The computer models from which this table is derived should be used for more specific planning applications. The table and deriving computer models should not be used for corridor or intersection design, where more refined techniques exist. Calculations are based on planning applications of the Highway Capacity Manual and the Transit Capacity and Quality of Service Manual.					
(Multiply motorized vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)						² Level of service for the bicycle and pedestrian modes in this table is based on number of motorized vehicles, not number of bicyclists or pedestrians using the facility.					
Paved Shoulder/Bicycle						³ Buses per hour shown are only for the peak hour in the single direction of the higher traffic flow.					
Lane Coverage	B	C	D	E		* Cannot be achieved using table input value defaults.					
0-49%	*	2,900	7,600	19,700		** Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached. For the bicycle mode, the level of service letter grade (including F) is not achievable because there is no maximum vehicle volume threshold using table input value defaults.					
50-84%	2,100	6,700	19,700	>19,700							
85-100%	9,300	19,700	>19,700	**							
PEDESTRIAN MODE²											
(Multiply motorized vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)											
Sidewalk Coverage	B	C	D	E							
0-49%	*	*	2,800	9,500							
50-84%	*	1,600	8,700	15,800							
85-100%	3,800	10,700	17,400	>19,700							
BUS MODE (Scheduled Fixed Route)³											
(Buses in peak hour in peak direction)											
Sidewalk Coverage	B	C	D	E							
0-84%	> 5	≥ 4	≥ 3	≥ 2							
85-100%	> 4	≥ 3	≥ 2	≥ 1							

Source:
Florida Department of Transportation
Systems Planning Office
www.dot.state.fl.us/planning/systems/sm/los/default.shtm